



## Standard Test Method for Shear Testing of Bulk Solids Using the Jenike Shear Cell<sup>1</sup>

This standard is issued under the fixed designation D 6128; the number immediately following the designation indicates the year of original adoption or, in the case of revision, the year of last revision. A number in parentheses indicates the year of last reapproval. A superscript epsilon ( $\epsilon$ ) indicates an editorial change since the last revision or reapproval.

### 1. Scope

1.1 This method<sup>2</sup> covers the apparatus and procedures for measuring the cohesive strength of bulk solids during both continuous flow and after storage at rest. In addition, measurements of internal friction, bulk density, and wall friction on various wall surfaces are included.

1.2 The most common use of this information is in the design of storage bins and hoppers to prevent flow stoppages due to arching and ratholing, including the slope and smoothness of hopper walls to provide mass flow. Parameters for structural design of such equipment may also be derived from this data.

1.3 *This standard does not purport to address all of the safety concerns, if any, associated with its use. It is the responsibility of the user of this standard to establish appropriate safety and health practices and determine the applicability of regulatory limitations prior to use.*

### 2. Terminology

#### 2.1 Definitions:

2.1.1 *angle of internal friction,  $\phi_i$* —the angle between the tangent to the yield locus and the abscissa.

2.1.2 *angle of wall friction,  $\phi'$* —the arctan of the ratio of the wall shear stress to the wall normal stress.

2.1.3 *bin*—a container or vessel for holding a bulk solid, frequently consisting of a vertical cylinder with a converging hopper. Sometimes referred to as silo, bunker, or elevator.

2.1.4 *bulk density,  $\rho$* —the mass of a quantity of a bulk solid divided by its total volume

2.1.5 *bulk solid*—an assembly of solid particles handled in sufficient quantities that its characteristics can be described by the properties of the mass of particles rather than the characteristics of each individual particle. May also be referred to as granular material, particulate solid, or powder. Examples are sugar, flour, ore, and coal.

2.1.6 *bunker*—synonym for bin, but sometimes understood as being a bin without any or only a small vertical part at the top of the hopper.

2.1.7 *consolidation*—the process of increasing the strength of a bulk solid.

2.1.8 *effective angle of friction,  $\delta$* —the inclination of the effective yield locus (EYL) as defined by Jenike.

2.1.9 *effective yield locus (EYL)*—straight line passing through the origin of the  $\sigma$ ,  $\tau$ -plane and tangential to the steady state Mohr circle, corresponding to steady state flow conditions of a bulk solid of given bulk density.

2.1.10 *elevator*—synonym for bin, commonly used in the grain industry.

2.1.11 *failure (of a bulk solid)*—plastic deformation of an overconsolidated bulk solid subject to shear, causing dilation and a decrease in strength.

2.1.12 *flow, steady state*—continuous plastic deformation of a bulk solid at critical state.

2.1.13 *flow function, FF*—the plot of unconfined yield strength versus major consolidation stress for one specific bulk solid.

2.1.14 *granular material*—synonym for bulk solid.

2.1.15 *hopper*—the converging portion of a bin.

2.1.16 *major consolidation stress,  $\sigma_1$* —the major principal stress given by the Mohr stress circle of steady state flow. This Mohr stress circle is tangential to the effective yield locus.

2.1.17 *Mohr stress circle*—the graphical representation of a state of stress in coordinates of normal and shear stress, that is, in the  $\sigma$ ,  $\tau$ -plane.

2.1.18 *normal stress,  $\sigma$* —the stress acting normally to the considered plane.

2.1.19 *particulate solid*—synonym for bulk solid.

2.1.20 *powder*—synonym for bulk solid, particularly when the particles of the bulk solid are fine.

2.1.21 *silo*—synonym for bin.

2.1.22 *shear stress,  $\tau$* —a stress acting parallel to the surface of the plane being considered.

2.1.23 *shear test*—an experiment to determine the flow properties of a bulk solid by applying different states of stress and strain to it.

2.1.24 *shear tester*—an apparatus for performing shear tests.

2.1.25 *time angle of internal friction,  $\phi_t$* —inclination of the time yield locus of the tangency point with the Mohr stress circle passing through the origin.

2.1.26 *time yield locus*—the yield locus of a bulk solid which has remained at rest under a given normal stress for a certain time.

<sup>1</sup> This testing method is under the jurisdiction of ASTM Committee D-18 on Soil and Rock and is the direct responsibility of Subcommittee D18.24 on Characterization and Handling of Powders and Bulk Solids.

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<sup>2</sup> This method is based on the "Standard Shear Testing Technique for Particulate Solids Using the Jenike Shear Cell," a report of the EFCE Working Party on the Mechanics of Particulate Solids. Copyright is held by The Institution of Chemical Engineers and the European Federation of Chemical Engineering.

2.1.27 *unconfined yield strength,  $f_c$* — the major principal stress of the Mohr stress circle being tangential to the yield locus with the minor principal stress being zero. A synonym for compressive strength.

2.1.28 *wall normal stress,  $\sigma_w$* — the normal stress present at a confining wall.

2.1.29 *wall shear stress,  $\tau_w$* —the shear stress present at a confining wall.

2.1.30 *wall yield locus*—a plot of the wall shear stress versus wall normal stress. The angle of wall friction is obtained from the wall yield locus as the arctan of the ratio of the wall shear stress to wall normal stress.

2.1.31 *yield locus*—plot of shear stress versus normal stress at failure. The yield locus (YL) is sometimes called the instantaneous yield locus to differentiate it from the time yield locus.

### 3. Significance and Use

3.1 Reliable, controlled flow of bulk solids from bins and hoppers is essential in almost every industrial facility. Unfortunately, flow stoppages due to arching and ratholing are common. Additional problems include uncontrolled flow (flooding) of powders, segregation of particle mixtures, useable capacity which is significantly less than design capacity, caking and spoilage of bulk solids in stagnant zones, and structural failures.

3.2 By measuring the flow properties of bulk solids, and designing bins and hoppers based on these flow properties, most flow problems can be prevented or eliminated.

3.3 For bulk solids with a significant percentage of particles (typically, one third or more) finer than about 6 mm ( $1/4$  in.), the cohesive strength is governed by the fines (-6-mm fraction). For such bulk solids, cohesive strength and wall friction tests may be performed on the fine fraction only.

### 4. Apparatus

4.1 The Jenike shear cell is shown in Figs. 1-3. It consists of a base (1), shear ring (2), and shear lid (3), the latter having a bracket (4) and pin (5). Before shear, the ring is placed in an offset position as shown in Fig. 1, and a vertical force  $F_v$  is applied to the lid and hence to the particulate solid within the cell by means of a weight hanger (6) and weights (7). A horizontal force is applied to the bracket by a mechanically driven measuring stem (8).

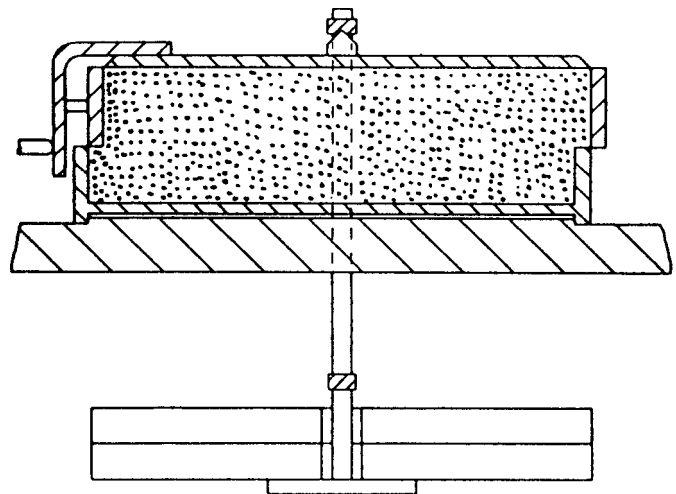


FIG. 2 Jenike Cell in Final Offset Position

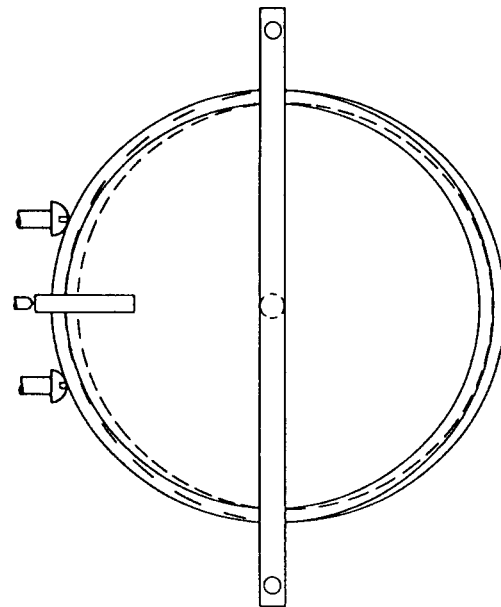


FIG. 3 Plan View of Jenike Cell Showing Offset

4.2 It is especially important that the shear force measuring stem acts on the bracket in the shear plane (plane between base and shear ring) and not above or below this plane.

4.3 The dimensions of the Jenike shear cells supplied by Jenike & Johanson, Inc. are given in the first two columns of the table in Fig. 4. These dimensions have been derived from English units. The standard size Jenike shear cell is made from aluminum or stainless steel, and a smaller 63-mm diameter cell made from stainless steel is also available. Since the actual dimensions are not believed to be critical, the same results could be obtained with a shear cell of the dimensions listed in the third column of the table in Fig. 4. However, it is important that the proportions of these dimensions be maintained approximately when using shear cells of different sizes. Besides the shear cell, the complete shear tester includes a force transducer which measures the shear force  $F_s$ , an amplifier and a recorder, a motor driving the force measuring stem, a twisting wrench, a weight hanger, a time consolidation bench, an

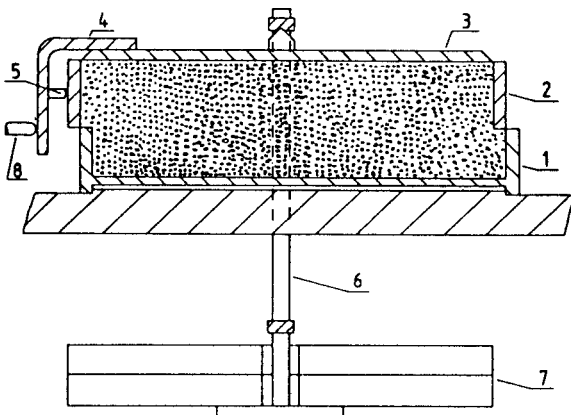


FIG. 1 Jenike Cell in Initial Offset Position

	JENIKE STANDARD	JENIKE SMALL SIZE	STANDARD SIZE
D/mm	95.25	63.5	95
H <sub>b</sub> /mm	12.7	9.525	13
H <sub>r</sub> /mm	15.875	11.113	16
H <sub>m</sub> /mm	9.525	7.938	10
T/mm	3 or greater	3 or greater	3 or greater
Material	Stainless Steel or Aluminum	Aluminum	Stainless Steel or Aluminum

GROOVES: 1 mm wide, 90° Included angle

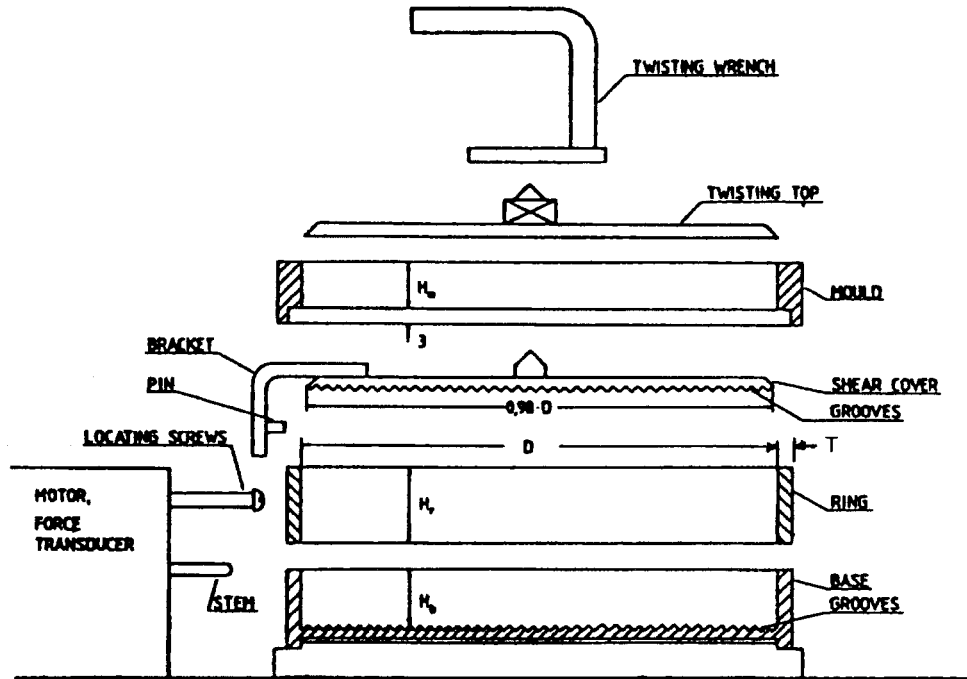


FIG. 4 Dimensions of the Jenike Cell

accessory for mounting wall material sample plates, and a calibrating device. The force transducer should be capable of measuring a force up to 500 N. The signal from the force transducer is conditioned by an amplifier and shown on a recorder. The motor driving the force measuring stem advances the stem at a constant speed in the range from 1 to 3 mm/min. The original Jenike shear tester has a speed of 2.72 mm/min when the power supply is 60 Hz. As an alternative to the twisting wrench, some shear testers are supplied with a twisting device in which the twist is applied by means of a shaft passing through bearings. In this way the likelihood of off-axis forces or extra forces being generated during twisting is minimized. Another alternative is to have the motor pull the force measuring stem instead of pushing it. When using any such alternative methods, it is essential that the user ensure that no measurement deviations are introduced.

4.4 The consolidation bench consists of several stations for time consolidation tests. One station is shown in Fig. 5. The station is equipped with a weight carrier (14) on which the weights may be placed and a flexible cover (15) to constrain the test cell and prevent any influence from environmental

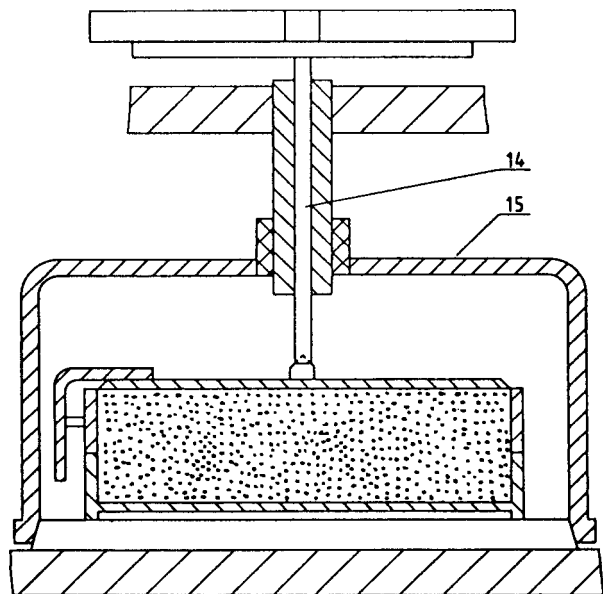


FIG. 5 Consolidating Bench Station

effects such as evaporation or humidification during time consolidation.

4.5 The arrangement for wall friction tests is shown in Fig. 6. For these tests it is convenient to have a special shear lid with a longer pin and bracket to permit a longer shear distance.

4.6 A device for calibrating the force transducer is shown in Fig. 7. It consists of a pivot (1) around which levers of equal length, (2) and (3) rotate. With counterweight (4) the device is balanced to have its neutral position as shown in the figure. The lever (2) exerts a force to the force measuring stem corresponding to the weights (5) which are hung on the lever (3). The calibration curve is used to convert the recorder reading to the applied shear force.

4.7 The laboratory used for powder testing should be free of vibrations caused by traffic or heavy machinery. Ideally the room should be temperature and humidity controlled, or, if this is not possible, it should be maintained at its nearly constant ambient conditions. Direct sunlight, especially on the time consolidation bench, is to be avoided.

## 5. Specimen Preparation

### 5.1 Filling the Cell (Fig. 8):

5.1.1 Place the shear ring on the base in the offset position shown in Fig. 1 and gently press the ring with the fingers against the locating screws (10) as shown in Fig. 3 and Fig. 9. Set these screws to give an overlap of approximately 3 mm for standard cell sizes and to ensure that the axis of the cell is aligned with the force measuring stem. Then place the mould ring (11) on the shear ring.

5.1.2 Fill the assembled cell uniformly in small horizontal layers by a spoon or spatula without applying force to the surface of the material until the material is somewhat over the top of the mould ring. The filling should be conducted in such a way as to ensure that there are no voids within the cell, particularly at "a" (Fig. 8) where the ring and the base overlap. Remove excess material in small quantities by scraping off with a blade (1). The blade should be scraped across the ring in a zig-zag motion. Take care not to disturb the position of the ring on the base. For scraping, a rigid sharp straight blade should be used, and, during scraping, the blade should be tilted as shown in Fig. 8.

### 5.2 Preconsolidation:

5.2.1 Place the twisting or consolidation lid (12) shown in Fig. 9 on the leveled surface of the material in the mould, then

place the hanger (6) on the twisting lid with weights (7) of mass  $m_{w_{tw}}$  being hung from the hanger. See Fig. 1. Lower the lid, hanger, and weights as slowly as possible to minimize aerated material being ejected from the cell.

NOTE 1—During this operation the material is compressed. With fine particulate solids it is necessary to wait until the vertical movement stops.

5.2.2 Remove the weights, hanger, and twisting lid. Fill and level the space above the compressed material as during filling.

NOTE 2—As will be mentioned later, this refilling procedure may not be necessary at all or may need to be performed several times, depending on the compressibility of the powder being tested. This operation determines what height of compacted material will have to be scraped off the ring after twisting.

### 5.3 Twisting:

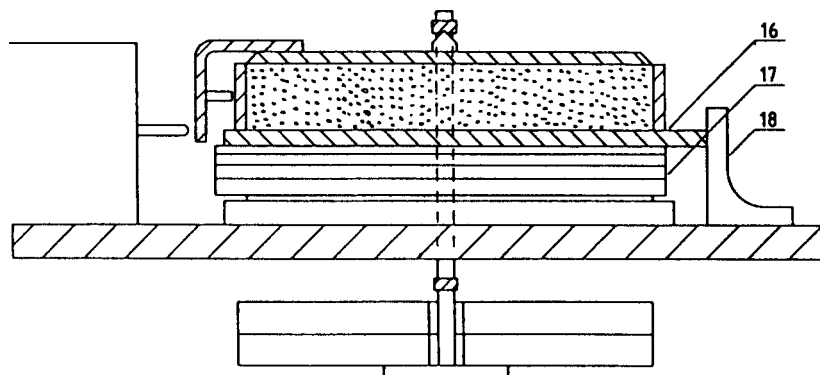
5.3.1 Place the twisting lid (12) with a smooth bottom surface on the leveled surface of material in the mould after filling or refilling. Place the hanger with weights of  $m_{w_{tw}}$  on the twisting lid. The weights on the hanger should correspond to a pressure of  $\sigma_{tw}$ , approximately equal to  $\sigma_p$ .

NOTE 3—If the surface of material in the cell is seen to be not level, then the filling procedure was not satisfactory and the filling operation will have to be repeated.

5.3.2 Having filled the cell, the twisting lid is usually twisted through 20 cycles by means of the twisting wrench (spanner) (13) or twisting device. Each twist consists of a 90° rotation of the lid which is then reversed. Care must be taken not to apply vertical forces to the lid during twisting. While twisting, press the ring against the locating screws with the fingers to prevent it from sliding from its original offset position.

NOTE 4—The mould and ring should be allowed to rotate freely and independently of each other. The rotation of the ring may be small but has an influence on the consolidation.

5.3.3 If the shear apparatus is not fitted with a special twisting device, the twisting is performed by holding the wrench in one hand and using the thumb and forefinger of the other to maintain the ring in the offset position against the locating screws (2) shown in Fig. 8. The twisting operation should be smooth and continuous, without jerks, and at the rate of about one twist per second. It is useful to mark the shear cell or twisting device to ensure a 90° rotation. After twisting, carefully remove the weights and hanger, then hold the lid in



**FIG. 6 Wall Friction Test**

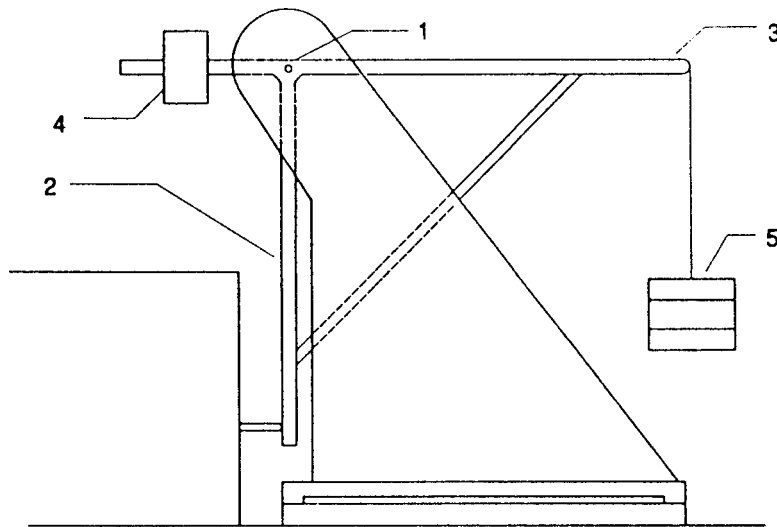


FIG. 7 Calibration Device

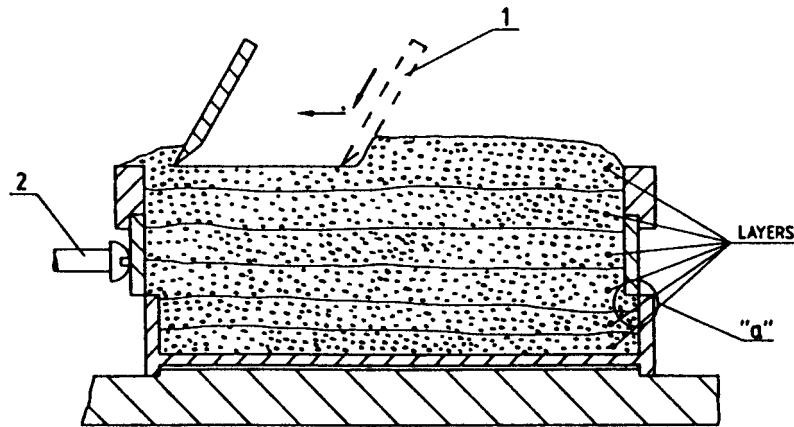


FIG. 8 Scraping Off Excess Powder

position by light finger pressure and carefully remove the mould. Slide the lid off the material in the cell, sliding it in the direction of the force measuring stem so that the shear ring is kept pressed in position against the locating screws.

NOTE 5—The compacted material above the ring should be evenly distributed if the filling has been satisfactory. The material remaining above the ring after twisting should be from 1 to about 3 mm thick. If after twisting the material surface is below the top of the ring, then it is necessary to prepare a new test specimen by using one more filling of the cell.

5.3.4 Scrape off excess material in small quantities to be flush with the top of the ring using a blade in the same way as that shown in Fig. 8. Do not exert downward force by the scraping blade.

NOTE 6—If coarse particles are present, scraping may tear them from the surface and alter the structure. In such cases, it is better to attempt to fill the cell so that the material surface is flush with the ring after consolidation. Care must again be taken not to displace the shear ring from its original offset position.

## 6. Procedure

### 6.1 Shear Testing Procedure:

#### 6.1.1 Synopsis:

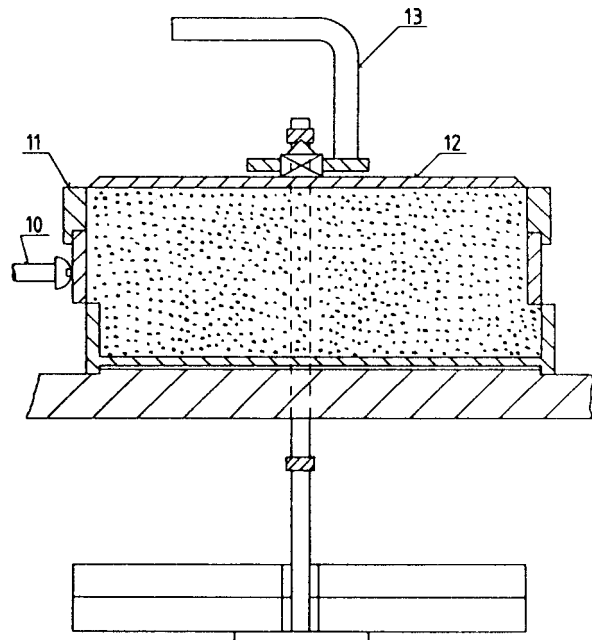


FIG. 9 Jenike Cell With Mould Ring and Consolidation Lid



6.1.1.1 Place the shearing lid centrally on the leveled surface of material with the pin of the bracket within 1 mm of the ring. Make sure that the bracket of the shear lid is in line with the force measuring stem. Place weights  $m_{Wp}$  corresponding to  $\sigma_p$  on the hanger, and gently lower the hanger with weights as slowly as possible onto the shear lid so as to not jar the specimen. Steady the hanger to prevent any visible swinging motion. Switch on the motor driving the force measuring stem, and perform a shear test for the full shear distance of approximately 6 mm from the offset position in Fig. 1 to the offset position in Fig. 2 for standard cell sizes. Record the shear force,  $F_s$ , for the whole shear distance.

NOTE 7—During shear, a shear zone develops in the specimen of particulate solid in the cell. Since the stem advances at a steady rate, the record of shear force versus time can be transformed into a shear force – shear strain plot.

6.1.1.2 Perform a shear test for the whole shear distance and inspect the shear force – shear strain plot. If the specimen is found to be underconsolidated, first increase the number of twists applied to the lid then increase the weight  $m_{Wtw}$  in accordance with A3.10. If the specimen is overconsolidated, first decrease the number of twists then reduce the weight  $m_{Wtw}$  in accordance with A3.11.

NOTE 8—In such a manner, it is possible by trial and error, to find a combination of weight,  $m_{Wtw}$ , and the number of twists so that for the selected weight,  $m_{Wp}$ , the shear force–shear strain plot indicates the presence of a critically consolidated specimen. This operation is called optimization. See Annex A3.

NOTE 9—Each shear test gives one point on a yield locus and consists of preshear and shear.

NOTE 10—The force measuring stem measures the shear force in the shear plane between the base and ring, and, hence, the corresponding normal force has to be determined in this plane. In the Jenike shear cell this normal force,  $F_v$ , is a vertical force produced by the combined masses of:

- Weights,  $m_W$
- Hanger,  $m_H$
- Shear Lid,  $m_L$
- Ring,  $m_R$
- Material in the shear ring above the shear plane,  $m_B$

NOTE 11—The shear ring is included in the vertical force since during shear the material dilates in the shear zone, as a result of which all material above the shear plane is lifted slightly. Since the material is constrained in the shear ring, any dilation of the cell contents brings about a lifting of the ring such that the weight of the ring is supported by the material in the ring rather than by the cell base. For preshear, this is not strictly so, because part of the weight of the ring may be transferred to the base. Therefore, because during preshear that portion of the weight of the ring transferred to the base is uncertain, the weight of the ring is included in the weights contributing towards the total normal force when calculating the preshear normal force. The influence of the ring-base contact on the shear and normal force can be avoided by carefully lifting the shear ring less than 1 mm and twisting it through a couple of degrees prior to shear while the shear lid has a weight applied to it.

### 6.1.2 Preshear:

6.1.2.1 The first part of the shear test consists of preparing a critically consolidated specimen by optimized twisting and then preshearing the specimen with a selected weight,  $m_{Wp}$ , to develop a shear zone in which steady state flow occurs.

6.1.2.2 Select the first preshear normal stress,  $\sigma_{p,1}$ , on the

basis of the bulk density of the test material, in accordance with the following table:

$\rho$ (kg/m <sup>3</sup> )	$\sigma_{p,1}$ (kPa)
< 300	approximately 1.5
300 to 800	approximately 2.0
800 to 1600	approximately 2.5
1600 to 2400	approximately 3.0
> 2400	approximately 4.0

6.1.2.3 A preliminary estimate of the bulk density can be made by placing the shear ring on a flat surface, packing the particulate solid in the ring with fingers, scraping the solid level with the top, and weighing the contained solid. From the weights and volume of the specimen, calculate the bulk density. Select higher preshear normal stress levels so that:

$$\begin{aligned}\sigma_{p,2} &= 2\sigma_{p,1} \\ \sigma_{p,3} &= 4\sigma_{p,1} \\ \sigma_{p,4} &= 8\sigma_{p,1}\end{aligned}$$

6.1.2.4 First perform tests using a preshear normal stress level of  $\sigma_{p,1}$  and then with increasing preshear normal stress levels. Select 3 to 5 shear normal stress levels within the range from 25 to 80 % of the preshear normal stress levels. Each measurement should be performed twice.

NOTE 12—Some adjustment in preshear normal stress levels may be necessary in order to cover the range of major consolidation stresses,  $\sigma_1$ , necessary to accurately calculate critical arching or ratholing dimensions, or both.

6.1.2.5 At the selected preshear normal stress prepare a nearly critically consolidated specimen and start preshear. The shear stress rises (Fig. 10) and attains the steady state value  $\tau_p$ . Maintain this shear stress in the shear cell through a relatively short shear distance (about 0.5 mm) to ascertain this value and then reverse the forward motion of the force measuring stem until the stem loses contact with the bracket (that is, the shear force falls to zero).

NOTE 13—The steady state shear stress  $\tau_p$  may be attained after relatively little shear, even before the shear ring and base completely overlap. With some materials a greater amount of shear may be necessary to attain steady state shear. However, the steady state shear stress should be attained after a maximum shear distance corresponding to three fourths of the total available. Constancy of the values of the steady state shear stress  $\tau_p$  obtained after preshear is an indication of the reproducibility of consolidation. With correctly consolidated samples, individual values of the steady state shear stress should not deviate by more than  $\pm 5\%$  from the average steady state shear stress for the given preshear normal stress. With some particulate solids, however, this tolerance cannot be achieved.

### 6.1.3 Shear:

6.1.3.1 Having attained a steady state flow condition, retract the force measuring stem causing the shear force to fall to zero (Fig. 10). For the second stage, replace the weight  $m_{Wp}$  by a smaller weight  $m_{Ws}$  and switch on the motor again to drive the measuring stem in the forward direction.

NOTE 14—When the stem touches the bracket, the shear force rapidly increases, goes through a maximum representing the yield shear force, and then begins to decrease. This part of the test is called shear.

NOTE 15—Shear may be continued until the whole overlap distance of the cell has been traversed in order to develop a distinct shear plane. The value  $\tau_s$  is the shear stress at failure (shear point) for the selected shear normal stress  $\sigma_s$  at the selected preshear normal stress  $\sigma_p$ . When reducing the normal stress before shear, it is recommended that weights be removed from the hanger until the required weight is left. If the test is to be carried out at low shear, and hence low normal stress levels, it may be necessary

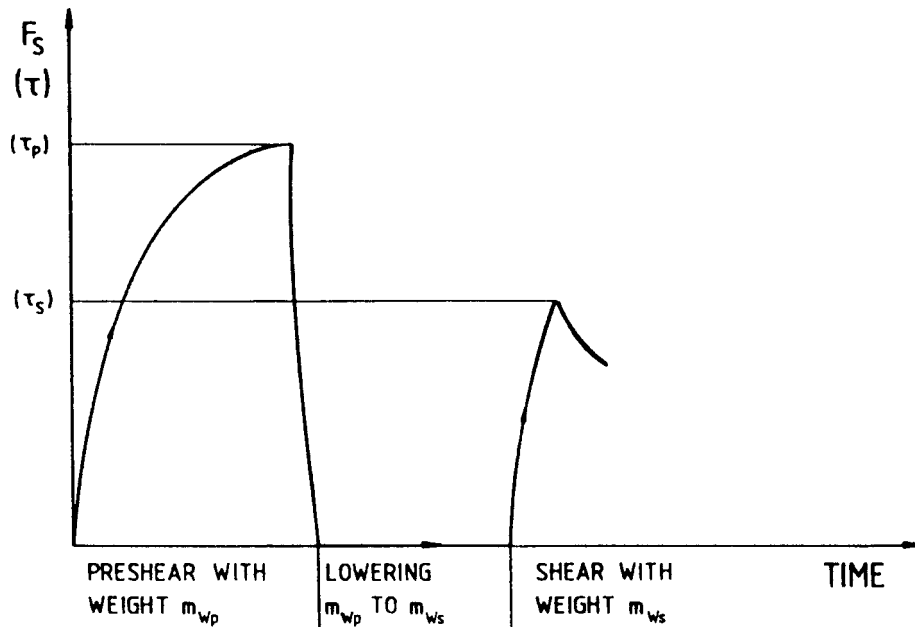


FIG. 10 Stress-Strain Curves — Preshear and Shear

to remove the hanger and place the weights directly on the lid. The weights should be removed and replaced in a gentle manner in whichever procedure is followed.

6.1.3.2 After each shear test, determine the overall bulk density of the specimen by weighing the specimen with the base, shear ring, and shear lid.

NOTE 16—Since the weights of base, ring, and lid are known and also the volume of the cell can be determined, the overall bulk density,  $\rho$ , of the specimen can be calculated. The value of the bulk density of the specimen after the shear test gives an indication of the reproducibility of specimen preparation.

6.1.3.3 After each shear test (and weighing), lift the shear ring with shear lid and material contained within the ring from the base and inspect the plane of failure.

NOTE 17—If the plane of failure cuts diagonally across the particulate solid either up to the shear lid or down to the bottom of the base, the test is invalid and will have to be repeated. If an invalid plane of failure persists, further tests at the given and lower shear normal stress levels cannot be performed and shear tests can be made only at higher shear normal stresses. In such a case, the intervals between the shear normal stress levels may have to be reduced to obtain the necessary minimum of three shear points on the yield locus. If the material is free flowing it may be impossible to observe the plane of failure.

6.1.4 Prorating:

6.1.4.1 Ideally all values of the preshear shear stress,  $\tau_p$ , for a given preshear normal stress would be identical. This would occur if the specimen was perfectly homogeneous, and specimen preparation completely repeatable. However, because of unavoidable experimental variation there is a scatter of  $\tau_p$

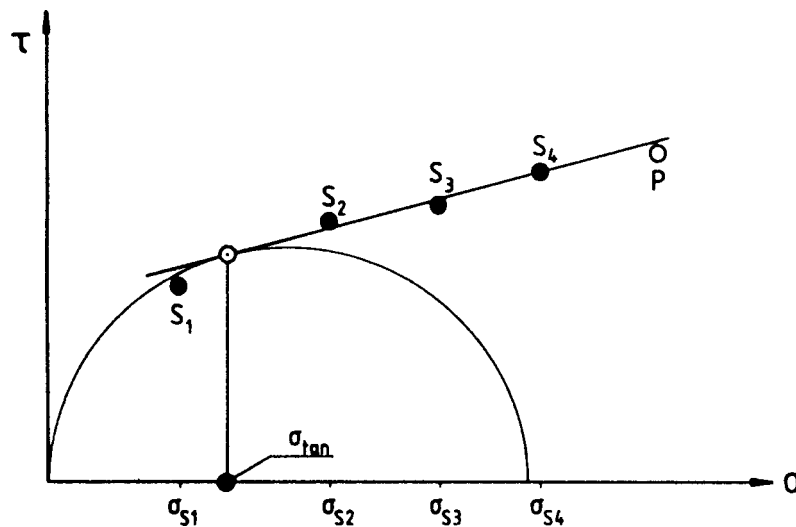


FIG. 11 Yield Locus and Data Points

values which affects the value of the shear stress,  $\tau_s$ .

6.1.4.2 To minimize the scatter, all measured shear stresses,  $\tau_s$ , may be corrected to take into account scatter in the preshear shear stresses,  $\tau_p$ . This empirical procedure is called prorating, and prorated values of  $\tau'_s$  of the measured values  $\tau_s$  are evaluated using the following equation.

$$\tau'_s = \tau_s \frac{\bar{\tau}_p}{\tau_p} \quad (1)$$

where  $\bar{\tau}_p$  = average of the preshear, shear stresses,  $\tau_p$ , of the corresponding preshear normal stress level (yield locus). Prorating assumes that variations in consolidation produce variations in shear stress,  $\tau_s$ , that are proportional to the corresponding variation in preshear shear stress,  $\tau_p$ .

6.1.5 Determination of Valid Shear Points:

6.1.5.1 For each consolidation condition, plot prorated and averaged shear points  $S_{1, i}$  of repeated measurements and the averaged preshear point  $P_i(\sigma_p, \bar{\tau}_p)$  on a  $\sigma, \tau$ -diagram (Fig. 11).

6.1.5.2 Adopt the following procedure to determine whether a yield point is valid.

6.1.5.3 Fit by means of a least squares fit a straight line called the yield locus,  $YL$ , to the three highest points  $S_2, S_3$ , and  $S_4$  (Fig. 11).

NOTE 18—If the straight line passes through or above Point  $P$  it can be used for further calculation. If, however, the straight line passes below Point  $P$  but the deviation in shear stress (between the steady state value and the extrapolated value based on the yield locus  $YL$ ) is less than 5 % (Fig. 12), it should be replotted to pass through Point  $P$  and refitted to the points  $S_2, S_3$ , and  $S_4$  (Fig. 13), and this new straight line should be used for further calculations. If the deviation is more than 5 %, either additional shear points should be run or the test should be redone at a different level of consolidation.

NOTE 19—From an inspection of the  $\sigma, \tau$ -diagram, it can be seen that the shear points on a yield locus are not equally spaced from zero normal stress to preshear normal stress, but begin at a certain minimum value of normal stress and end some distance before the preshear normal stress is reached. Considering the situation in more detail, Fig. 14 shows one yield locus with a preshear point  $P$  and four valid shear points,  $S_1$ – $S_4$ . One Mohr circle, 1, (the steady state Mohr circle) is drawn through the preshear Point  $P$  and tangentially to the extrapolated yield locus (the point of tangency is shown on Fig. 14 as  $B$  and defines the end point of the yield

locus).<sup>3</sup> A second Mohr circle, 2, (the unconfined strength Mohr circle) is drawn, passing through the origin and tangential to the extrapolated yield locus (this point of tangency is denoted by  $A$  in Fig. 14). Valid yield points must lie between the points of tangency  $A$  and  $B$ .

NOTE 20—Points to the left of Point  $A$  are invalid because they represent a state where tensile stresses can occur in the shear cell. This can be seen by considering the yield point on Fig. 14 marked by  $S_{(-)}$ , below Point  $A$ . If a Mohr circle 3 is drawn through this point, which is tangential to the extrapolated yield locus, part of that circle will lie to the left of the origin indicating negative normal stresses, that is, tensile stresses.

6.2 Shear Testing Procedure for Time Consolidation:

6.2.1 When a particulate solid is exposed to a normal or compressive stress for some time it may gain strength. This gain in strength may be measured in the Jenike shear cell, and the effect is called time consolidation.

6.2.2 To gain an understanding of time consolidation consider the following experiment. A critically consolidated specimen is prepared and presheared with weight  $m_{wp}$ . After attaining steady state flow the advance of the force measuring stem is stopped but the stem is not retracted. The shear zone formed thus remains under the normal and shear stresses corresponding to steady state flow and is kept in this state for a definite time,  $t$ . If the stem is then retracted, the shear force will drop to zero, and the actual shear test may be performed in the usual way. It is found that with materials which gain strength during time consolidation, a higher shear strength will be measured. In a  $\sigma, \tau$ -diagram, the time yield locus for time consolidation will lie above the instantaneous flow yield locus.

6.2.3 If the effect of time consolidation in the Jenike shear cell were measured as previously described, one test would monopolize the shear cell for a very long time. Also creep of the specimen could cause a decrease in the applied shear force during the resting phase.

<sup>3</sup> This method of constructing the steady state Mohr circle is specified by Jenike, A. W., *Storage and Flow of Solids*, Bulletin 123, Utah Engineering Experiment Station, 1964 (Rev. 1980) and by the *Standard Shear Testing Technique for Particulate Solids using the Jenike Shear Cell*, a report of the EFCE Working Party on the Mechanics of Particulate Solids. Copyright for the latter is held by The Institution of Chemical Engineers and the European Federation of Chemical Engineering, Rugby, England, 1989. Alternative methods of construction have been proposed. See, for example, Peschl, I.A.S.Z., "Measurement and Evaluation of Mechanical Properties of Powders," *Powder Handling and Processing*, Vol 1, No. 2, June 1989, pp. 135–142.

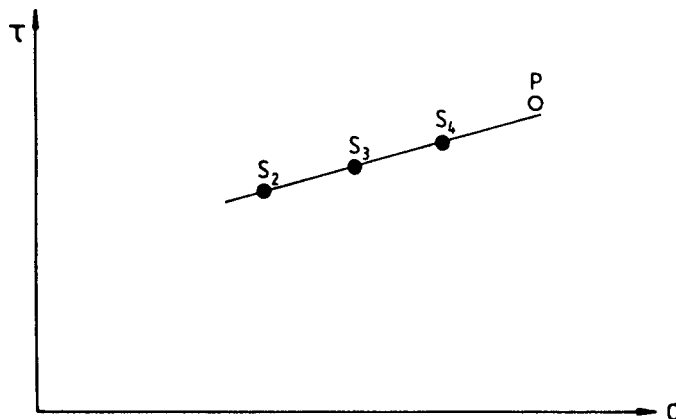


FIG. 12 End Point Above Fitted Line



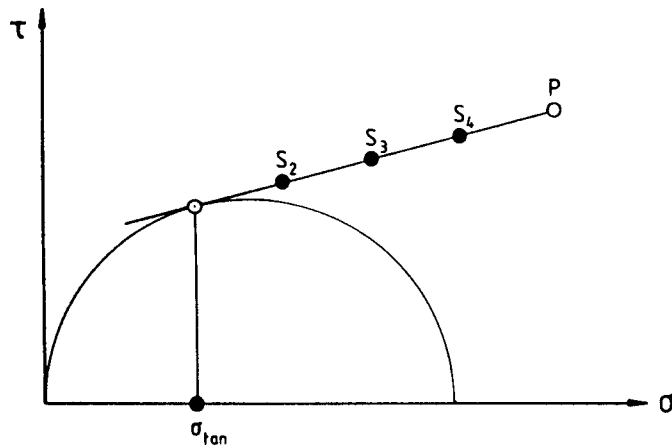


FIG. 13 End Points on Fitted Line

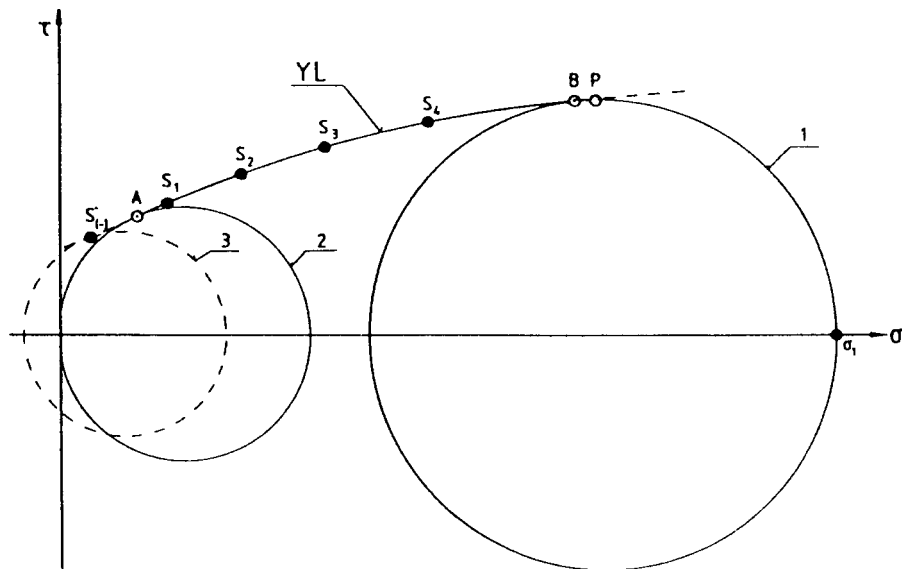


FIG. 14 Yield Locus Showing Valid Shear Points

6.2.4 Time consolidation tests are therefore carried out using a consolidating bench which consists of several shear cells which can be independently loaded.

6.2.5 *Specimen preparation and preshear time effects*—After completion of instantaneous testing and evaluation, perform time tests at the same preshear normal stress levels.

NOTE 21—For a selected preshear normal stress, specimen preparation and preshear are the same as for the instantaneous test.

6.2.6 *Time Consolidation:*

6.2.6.1 Perform the test for time consolidation in the following way. Using the shear tester, prepare and preshear samples with weight  $m_{wp}$  in the normal manner and then retract the stem after preshear. Remove the hanger with weights. Then transfer the shear cells (base, shear ring, shear lid, and material) to the consolidating bench. In order to prevent the evaporation or take up of moisture from the ambient environment, place a flexible cover over each cell, and then load each by placing a weight  $m_{wt}$  either directly on the lid or by means of a loading rod.

NOTE 22—When the shear cell is transferred from the shear tester to the

consolidating bench, care should be taken that the ring is not moved relative to the base. As the weight carrier is lowered on the shear lid, great care must be taken in adjusting the position of the shear cell on the consolidating bench to ensure that the weight carrier acts centrally on the shear lid or on a similarly sized compression plate when the weight carrier is lowered.

6.2.6.2 Select the weight  $m_{wt}$  in such a way that the stress state in the specimen during time consolidation is the same as during preshear (that is, steady state flow).

NOTE 23—During preshear a normal stress as well as a shear stress is acting, although on the consolidating bench only normal stresses can be applied. The stress state developed by the application of normal stress alone, however, can closely approximate that developed in steady state flow. The Mohr circle shown in Fig. 14 is drawn through Point P (steady state flow) and is tangential to the yield locus. During time consolidation, the specimen is loaded with the major principal stress,  $\sigma_1$ , of that Mohr circle as shown in Fig. 14.

6.2.6.3 Calculate the mass of the weights to be placed on the weight carrier from:

$$m_{wt} = \frac{A \times \sigma_1}{g} - m_c - m_r - m_l - m_b \quad (2)$$

where  
 $m_c$  = mass of the weight carrier.

NOTE 24—Since the shear strength after time consolidation is not very sensitive to the force  $\sigma_1$ , it is sufficient to select  $m_{wt}$  to satisfy Eq 2 to within  $\pm 5\%$ .

6.2.6.4 After the chosen time,  $t$ , has elapsed, remove the weights from the weight carrier, raise the flexible cover, raise the weight carrier, and transfer the shear cell to the shear tester.

6.2.7 *Shear of Specimen After Time Consolidation*—Select a weight  $m_{ws}$ . Perform shear in the same manner as for instantaneous flow. For time tests, select no more than three shear normal stress levels for each preshear stress.

NOTE 25—Due to the scatter obtained in time shear tests, it is recommended that they be performed at least twice.

6.2.8 *Validity of Time Shear Points*—Plot the time shear points in  $\sigma, \tau$ -coordinates (Fig. 15) and draw a straight line called the time yield locus, *TYL*, through the highest shear point and parallel to the instantaneous yield locus (for that particular preshear normal stress level). Draw a Mohr circle through the origin and tangential to this straight line.

NOTE 26—Those time shear points which lie to the right of this point of tangency  $A$ , of the Mohr circle to the straight line time yield locus are considered valid. The normal stress applied at shear for the highest time yield point  $S_{3t}$  is generally less than the normal stress applied at the end point,  $B$ , of the instantaneous yield locus.

6.3 *Procedure for Wall Friction:*

6.3.1 When measuring the friction between the particulate solid and a coupon of silo wall material in a wall friction test, replace the base of the shear cell by the coupon of wall material. Shear the specimen contained in the upper part of the shear cell (the ring and shear lid) over the wall material coupon under different wall normal stresses  $\sigma_w$  and measure the resulting wall shear stresses  $\tau_w$ .

6.3.2 *Selection of Wall Friction Normal Stress Levels*—Select six wall friction normal stress levels,  $\sigma_{w1}$  to  $\sigma_{w6}$ , where  $\sigma_{w1}$  is the smallest normal stress. The largest normal stress,  $\sigma_{w6}$ , should be approximately equal to the major consolidation stress,  $\sigma_{1,2}$ , of the second preshear normal stress,  $\sigma_{p,2}$ . The smallest normal stress  $\sigma_{w1}$  will normally include the hanger without weights.

6.3.3 *Wall Coupon and Material Specimen Preparation:*

6.3.3.1 The coupon of wall material should be approximately 120 by 120 mm for use with the standard size shear cell. Wash the wall material coupon and dry thoroughly before the test. Do not touch the surface after washing by the bare hands. Rub the particulate solid under test onto the surface of the wall coupon by hand, wearing gloves.

6.3.3.2 Shim (17) the wall coupons (16) (see Fig. 6) so that the top surface of the coupon is the horizontal plane of the force measuring stem. Place the ring on the wall coupon and set it against the locating screws. Adjust the position of the wall coupon so that it just covers the inside of the shear ring on the stem side and permits maximum travel of the ring over the coupon during the test. Fix the position of the wall coupon (18).

6.3.3.3 Place the mould ring on the shear ring, and fill the shear ring and mould ring with the particulate solid. Scrape excess material flush with the top of the mould ring.

6.3.3.4 Place the twisting lid on the leveled material, place the hanger on the lid, and place weights on the hanger, corresponding to the normal stress,  $\sigma_{w6}$ . Using the twisting wrench, twist the lid to homogenize the specimen. Do not apply vertical stress to the twisting lid by the twisting wrench. During twisting allow the mould ring and the shear ring to rotate. After consolidation, carefully remove the weight hanger and weights from the twisting lid. Hold the twisting lid down lightly with the fingers and remove the mould ring. Carefully remove the twisting lid from the cell by sliding towards the locating screws, and scrape off the caked material level with the top of the shear ring. Observe the same procedure and precautions as for preparation of a specimen for shear testing.

6.3.3.5 If after consolidation, the level of the compressed material is below the top of the shear ring, refill the cell as previously described prior to removing the mould ring.

6.3.4 *Wall Friction Tests:*

6.3.4.1 Stack weights on the hanger corresponding to the wall friction normal stress  $\sigma_{w6}$ . Include the weight of the hanger in the calculation of  $\sigma_w$ . Then place the shear lid on the leveled material in the shear ring and place the hanger on the lid. Select the weights in such a way that by removing a weight (or weights) the normal stress is reduced stepwise from  $\sigma_{w(i+1)}$

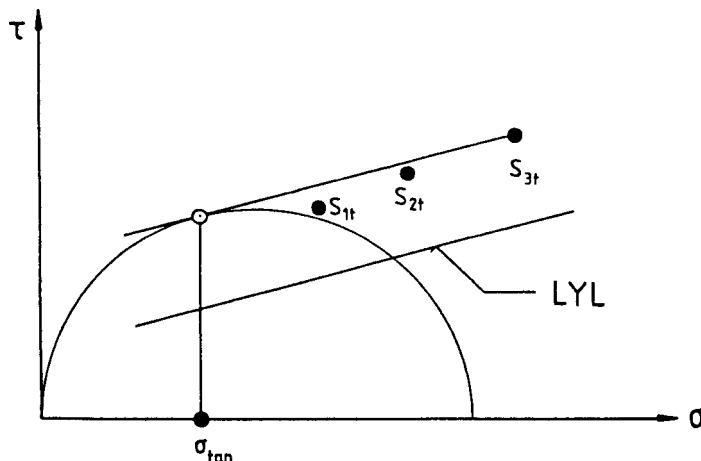


FIG. 15 Validity of Points on the Time Yield Locus

to  $\sigma_{wi}$ . Twist and manually lift the ring slightly off the wall material coupon to prevent it from dragging on the wall coupon. Then switch on the motor driving the force measuring stem.

NOTE 27—As the shear starts, the shear stress will begin to rise. It will approach a steady state either directly or may pass through a maximum.

6.3.4.2 When the shear stress,  $\tau_{w6}$ , reaches a constant value, remove weight(s) until the normal stress is reduced to  $\sigma_{w5}$ . Continue to advance the stem during removal of the weights. The shear stress,  $\tau_{w6}$ , is sometimes ignored. When the shear stress has again reached a constant value, record the shear stress,  $\tau_{w5}$ , and remove more weights to reduce the normal stress to  $\sigma_{w4}$ . When the shear stress has again become constant, record the stress  $\tau_{w4}$ . Continue this procedure over the range of selected normal stresses.

6.3.4.3 If the stem has reached the limit of its travel before the whole range of required normal stresses has been tested (say at normal stress  $\sigma_{wi}$ ), retract the stem, carefully push back the ring to the locating screws, increase the normal stress to  $\sigma_{w(i+1)}$  and continue testing, ignoring the first (repeated) reading of  $\tau_{w(i+1)}$ .

6.3.4.4 On completion of the tests, weigh the specimen to determine  $m_B$ .

6.3.4.5 Repeat wall friction tests two to three times with new samples of the particulate solid.

NOTE 28—Sometimes there will be a rapid oscillation of the indicated shear force because of slip-stick behavior. The shear stress maxima recorded during shear will be used to evaluate the wall friction angle  $\phi'$ .

NOTE 29—In many cases there is no distinct difference between static and kinematic friction. However, the shear force may pass through a maximum when starting a wall friction test, that is, there is a peak shear stress at  $\tau_{w6}$ . If static friction is suspected, the static angle of wall friction can be determined as follows: A test is performed as previously described, but when the shear force has passed through the maximum the stem is retracted. After the shear force has fallen to zero, the weight on the hanger is reduced and the motor is started again. The shear force will again pass through a maximum, and the procedure of retracting the stem and reducing the weight is repeated. The peak values of  $\tau_w$  are used to evaluate the static angle of wall friction.

#### 6.4 Wall Friction Time Tests:

6.4.1 Static wall friction tests with time consolidation are also known as adhesion tests.

6.4.2 Cut three coupons of the same wall material to fit under the covers of the consolidating bench and wash and dry them thoroughly.

6.4.3 Perform a wall friction test using wall friction normal stresses,  $\sigma_{w6}$  to  $\sigma_{w1}$ , to obtain a defined compaction of the particulate solid particles. Retract the stem and push back the shear ring against the locating screws. Increase the load to  $\sigma_{w6}$  and perform a shear test until the shear stress attains a constant value. Without stopping, remove shear weights to obtain the stress  $\sigma_{w5}$ . When the shear stress again reaches a constant value, stop and retract the stem.

NOTE 30—This step can be considered as wall friction 'preshear' which gives the 'initial' shear stress  $\tau_{wp5}$ .

6.4.4 Remove the weights and hanger and very carefully place the wall coupon with material specimen, shear ring, and shear lid onto the consolidating bench under the cover.

NOTE 31—At this time, the material specimen will have little or no adhesion to the wall plate and may move slightly. This however does not negate the test.

6.4.5 Using the weight carrier or hanger with appropriate weights, apply the normal stress,  $\sigma_{w5}$ . If a weight carrier is used calculate the appropriate weights required using Eq 2.

6.4.6 After the chosen time,  $t$ , has elapsed, transfer the wall coupon with material specimen, shear ring, and shear lid to the shear tester. Take care not to bump the specimen during this transfer as any break in the adhesive bond will nullify the test. Using the weight hanger and weights, load the shear lid to give a normal stress  $\sigma_{w5}$  and perform shear in the normal way. The shear stress will pass through a maximum, the 'time' wall friction shear stress, and is given the symbol  $\tau_{wt5}$ .

6.4.7 The pair of stresses ( $\sigma_{w5}$ ,  $\tau_{wt5}$ ) define Point  $S_{w5}$ . Using the second wall coupon, obtain another point ( $\sigma_{w3}$ ,  $\tau_{wt3}$ ) by preshearing the specimen under normal stresses of  $\sigma_{w4}$  and  $\sigma_{w3}$  and time consolidate it at  $\sigma_{w3}$  as previously described. Obtain a third point ( $\sigma_{w1}$ ,  $\tau_{wt1}$ ) using the normal stresses,  $\sigma_{w2}$  and  $\sigma_{w1}$ , for preshear and  $\sigma_{w1}$  for time consolidation. Further points ( $\sigma_{w4}$ ,  $\tau_{wt4}$ ) and ( $\sigma_{w2}$ ,  $\tau_{wt2}$ ) can be measured using the same procedure.

### 7. Calculation or Interpretation of Results

#### 7.1 Data Processing for Instantaneous Shear Tests:

7.1.1 Evaluate results separately for every chosen value of the preshear normal stress, although all points should be shown on one  $\sigma, \tau$ -diagram.

7.1.2 Plot the preshear point,  $P$ , and all valid shear points for one given preshear normal stress level in  $\sigma, \tau$ -coordinates. Draw a smooth line through the valid points and extrapolate it to the preshear normal stress. If this line passes above or through Point  $P$ , use it for further calculations. If it passes below Point  $P$ , plot a new line passing through Point  $P$  and fit it to all the valid yield points.

7.1.3 Draw a Mohr circle through the origin, tangential to this smooth line, the instantaneous yield locus (YL in Fig. 16).

NOTE 32—The higher point of intersection of this Mohr circle with the  $\sigma$ -axis is the unconfined yield strength,  $f_c$ .

7.1.4 Draw a second Mohr circle through Point  $P$ , tangential to the smooth line in such a way that the point of tangency is to the left of the preshear Point  $P$ .

NOTE 33—The upper point of intersection of this Mohr circle with the normal stress axis is the major consolidation stress,  $\sigma_1$ . In this way, the pair of values,  $f_c$  and  $\sigma_1$ , associated with this particular yield locus are produced, these values all being associated with the major consolidation stress  $\sigma_1$ .

NOTE 34—The yield locus is normally found to show a small curvature, convex upwards. With many particulate solids a straight line is a sufficient approximation. If the yield locus is approximated as a straight line for all particulate solids, then subsequent calculations are much simpler, but, in some cases, somewhat conservative results may be obtained, that is, a higher  $f_c$  value will be determined than when using a fitted curve.

7.1.5 Determine the angle of internal friction of the particulate solid,  $\phi_i$ , at the major consolidation stress,  $\sigma_1$ , by measuring the angle between a yield locus and the  $\sigma$ -axis.

NOTE 35—Since this angle varies with  $\sigma$  when using a smooth line yield locus, its value should be read from the linearized yield locus ( $LYL$ ), which is the tangent to the two Mohr circles characterizing the major

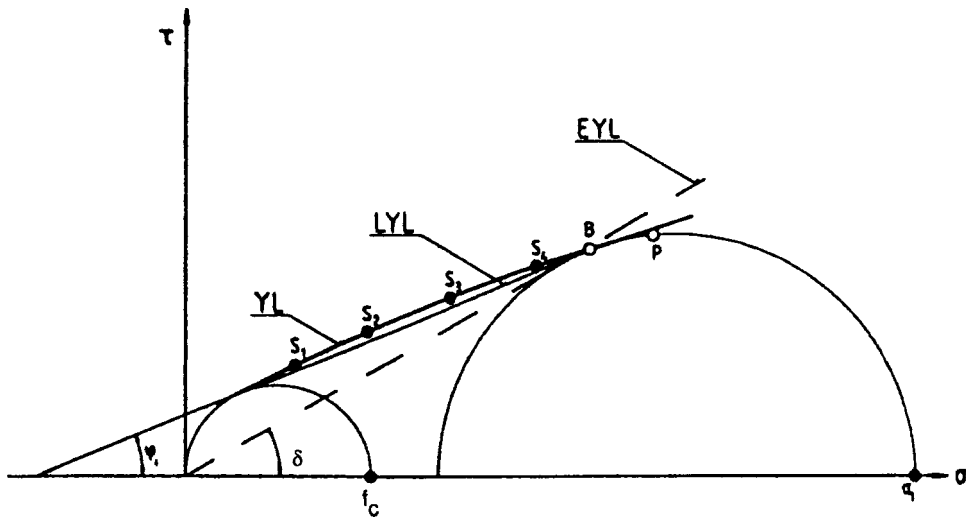


FIG. 16 Mohr Circles, Angles of Friction and Yield Loci

principal stresses  $\sigma_1$  and  $f_c$  (Fig. 16).

7.1.6 Draw a straight line through the origin, tangential to the major principal stress Mohr circle. This line, which is the effective yield locus (EYL), forms an angle  $\delta$  with the axis, called the effective angle of friction. For a given preshear normal stress and value of  $\sigma_1$ , determine a mean bulk density,  $\rho$ .

NOTE 36—The preceding calculation produces values of  $f_c$ ,  $\phi$ ,  $\delta$ , and  $\rho$  for a given  $\sigma_1$ . By making measurements at several preshear normal stresses, the dependencies of  $f_c$ ,  $\phi$ ,  $\delta$ , and  $\rho$  on  $\sigma_1$  can be determined. The dependency of  $f_c$  on  $\sigma_1$  is called the flow function (FF) for instantaneous flow.

7.2 Evaluation of Time Shear Test Data:

7.2.1 Carry out evaluations separately for each preshear normal stress level. Plot the valid time shear points for each preshear normal stress level in  $\sigma, \tau$ -coordinates (Fig. 17). Fit a smooth line through the points. This smooth line is called the time yield locus.

7.2.2 Draw a Mohr circle through the origin and tangential to the time yield locus.

NOTE 37—The highest point of intersection of this Mohr circle with the  $\sigma$ -axis is the time unconfined yield strength,  $f_{ct}$ . This value, together with

the major consolidation stress for instantaneous flow,  $\sigma_1$ , for each selected preshear normal stress gives the values  $\sigma_1, f_{ct}$  that are used in plotting the time flow function,  $FF_t$ .

NOTE 38—The angle between the time yield locus and the  $\sigma$ -axis is the time angle of internal friction,  $\phi_t$  for that particular  $\sigma_1$  (Fig. 17).

7.3 Evaluation of Wall Friction Test Data:

7.3.1 Plot the points  $\sigma_{wi}, \tau_{wi}$  on  $\sigma, \tau$ -coordinates and draw a smooth line through the points (Fig. 18).

NOTE 39—This is the wall yield locus (WYL) of the particulate solid on the specific wall material. The plot of the WYL will be a straight line or a curve convex upwards.

7.3.2 If the wall yield locus is a straight line passing through the origin, then  $\phi' = \text{constant}$ . Otherwise, superimpose a steady state flow Mohr circle associated with a yield locus and a major consolidation stress,  $\sigma_1$ , on the WYL. Determine the upper point of intersection of the WYL with the steady state flow Mohr circle and draw a straight line through the origin and this point of intersection. The angle that this straight line subtends with the  $\sigma$ -axis is the kinematic angle of wall friction  $\phi'$  at this particular major consolidation stress  $\sigma_1$ .

7.3.3 By repeating the procedure with consolidating Mohr circles associated with higher preshear normal stresses, obtain

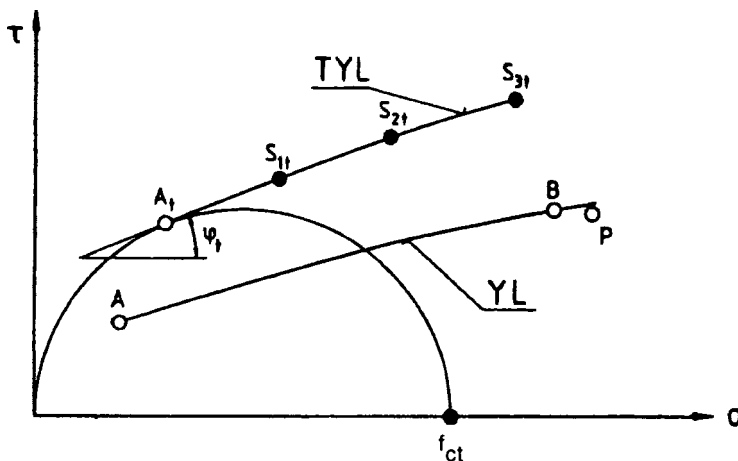


FIG. 17 Time Yield Locus

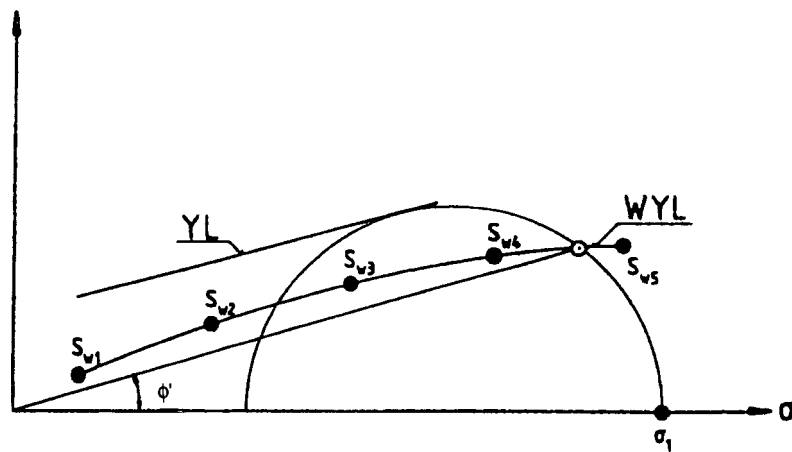


FIG. 18 Wall Yield Locus

the corresponding values  $(\sigma_1, \phi')$  for each preshear normal stress.

7.3.4 Obtain the static angle of wall friction  $\phi'_s$  by using the  $\sigma_{wi}, \tau_{wi}$  values of the peaks. The steady state values give the kinematic angle of wall friction  $\phi'$ .

7.4 Evaluation of wall friction time test data—Evaluate wall friction time tests in a similar way to kinematic wall friction tests. Plot the points  $S_{wt}$  on  $\sigma, \tau$ -coordinates and fit them by a smooth line called the time wall yield locus (TWYL). The analysis gives a time angle of wall friction  $\phi'_t$  for each of the  $\sigma_1$  values of the superimposed steady state Mohr circles.

## 8. Report

8.1 Analysis of shear test data for instantaneous flow and after time consolidation and wall friction tests provides the dependence on  $\sigma_1$  of the following properties:

8.1.1 Unconfined yield strength,  $f_c$ , that is, flow function,  $FF$ ,

8.1.2 Time unconfined yield strength,  $f_{ct}$ , that is, time flow function  $FF_t$ ,

8.1.3 Effective angle of friction,  $\delta$ ,

8.1.4 Angle of internal friction,  $\phi_i$ , for instantaneous flow,

8.1.5 Angle of internal friction,  $\phi_t$ , after time consolidation.

8.1.6 Angle of kinematic wall friction,  $\phi'$ ,

8.1.7 Angle of static wall friction,  $\phi'_s$ ,

8.1.8 Angle of time wall friction,  $\phi'_t$ , and

8.1.9 Bulk density,  $\rho$ .

8.2 The preceding dependencies are illustrated graphically

in Fig. 19. The flow function for instantaneous flow,  $FF$ , is obtained by fitting a smooth curve through the pairs of points  $(\sigma_1, f_c)$ . See Fig. 19(e). The  $\sigma_1$  and  $f_c$  coordinates should be to the same scale. The flow function usually has a slight curvature convex upwards.

8.3 In a similar way, the time flow function,  $FF_t$ , is obtained by fitting a smooth curve or a straight line to the pairs  $(\sigma_1, f_{ct})$ .

8.4 The effective angle of friction,  $\delta$ , is plotted on Fig. 19(d) by fitting a smooth curve through the points  $(\sigma_1, \delta)$ . For cohesive materials,  $\delta$  will increase with decreasing  $\sigma_1$ .

8.5 In a similar way, the remaining properties can be plotted,  $\phi_i$  and  $\phi_t$  on Fig. 19(c) and all the angles of wall friction on Fig. 19(b) and  $\rho$  on Fig. 19 (a).

## 9. Precision and Bias

9.1 Precision—Data are being evaluated to determine the precision of this test method. In addition, Subcommittee D18.24 is seeking pertinent data from users of this test method.

9.2 Bias—There is no accepted reference value for this test method; therefore, bias cannot be determined.

## 10. Keywords

10.1 bulk solid; cohesive strength; effective angle of friction; effective yield locus; flow function; Jenike shear cell; kinetic wall friction angle; internal friction angle; powder; unconfined yield strength; wall friction



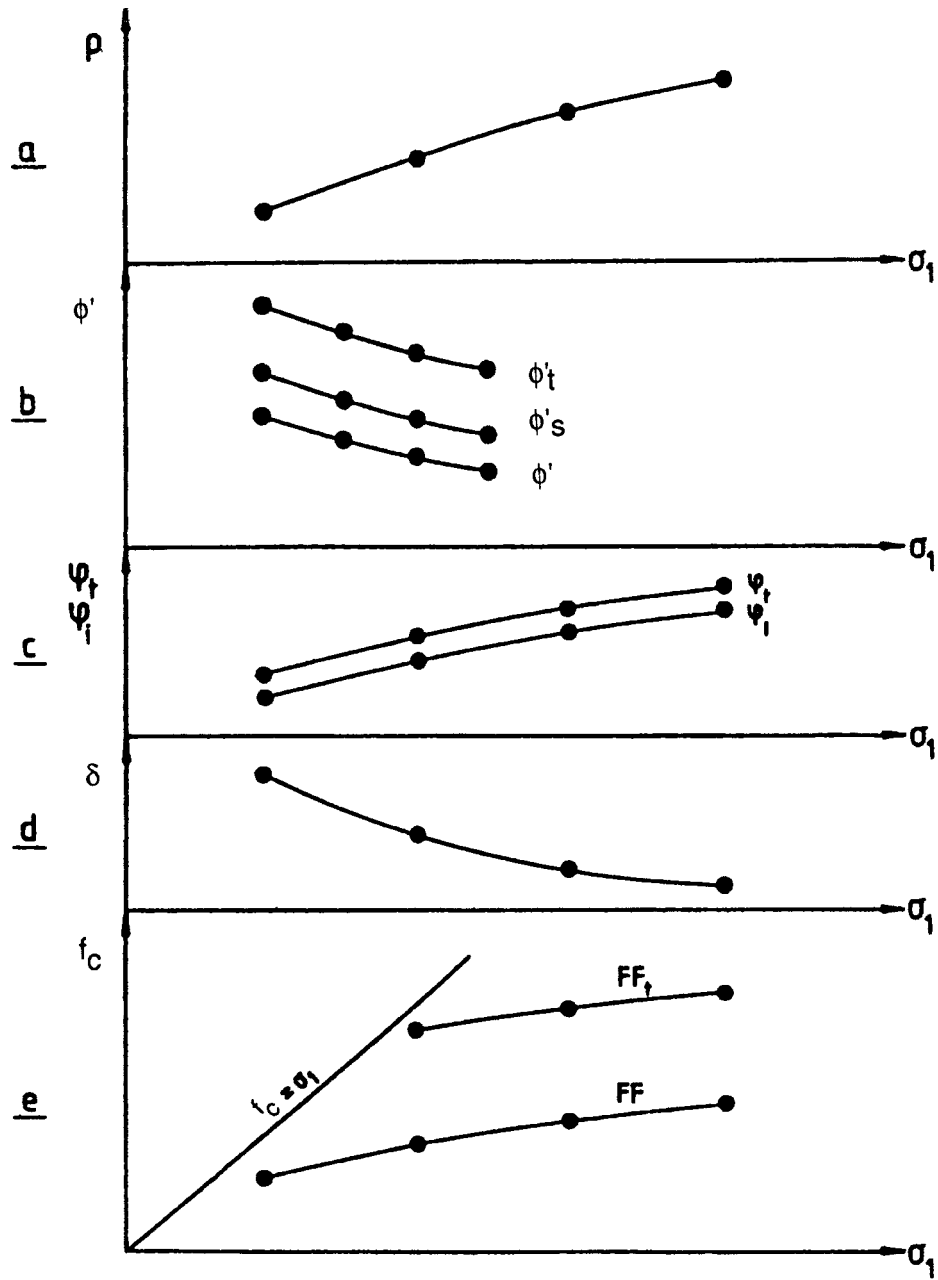


FIG. 19 Powder Properties as a Function of  $\sigma_1$

ANNEXES

(Mandatory Information)

A1. List of Symbols

$A$	cross-sectional area of cell, $m^2$	$m_H$	mass of hanger, kg
$F_v$	vertical force, N	$m_L$	mass of shear lid, kg
$F_s$	shear force, N	$m_M$	total mass of particulate solid in shear cell, kg
$f_c$	unconfined yield strength, $N/m^2$	$m_R$	mass of shear ring, kg
$f_{ct}$	time unconfined yield strength, $N/m^2$	$m_W$	mass of weights, kg
$g$	acceleration due to gravity ( $g = 9.81 \text{ m/s}^2$ ), $m/s^2$	$m_{WP}$	mass of weights during preshear, kg
$m_B$	mass of particulate solid in shear ring, kg	$m_{Ws}$	mass of weights during shear, kg
$m_C$	mass of weight carrier on time consolidation bench, kg	$m_{Wt}$	mass of weights during time consolidation, kg
		$m_{Wtw}$	mass of weights during twisting, kg

$T$	thickness of shear ring, mm	$\sigma_{tw}$	normal stress during twisting, N/m <sup>2</sup>
$t$	consolidation time, h	$\sigma_w$	wall normal stress, N/m <sup>2</sup>
$\delta$	effective angle of friction, °	$\rho$	bulk density, kg/m <sup>3</sup>
$\phi'$	kinematic angle of wall friction, °	$\tau$	shear stress, N/m <sup>2</sup>
$\phi's$	static angle of wall friction, °	$\tau_p$	shear stress at preshear, N/m <sup>2</sup>
$\phi't$	time angle of wall friction, °	$\tau_s$	shear stress at failure (shear point), N/m <sup>2</sup>
$\phi_i$	angle of internal friction, °	$\tau's$	prorated shear stress at failure, N/m <sup>2</sup>
$\phi_t$	time angle of internal friction, °	$\tau_w$	wall shear stress, N/m <sup>2</sup>
$\sigma$	normal stress, N/m <sup>2</sup>	$\tau_{wp}$	initial shear stress in wall friction time test, N/m <sup>2</sup>
$\sigma_p$	preshear normal stress, N/m <sup>2</sup>	$\tau_{wt}$	time wall shear stress, N/m <sup>2</sup>
$\sigma_s$	shear normal stress or normal stress at shear, N/m <sup>2</sup>		

## A2. Selection of Sample, Shear Cell, and Test Weights

### A2.1 Sample Selection:

A2.1.1 For meaningful results, select a representative sample of the particulate solid with respect to moisture content, particle size distribution, and temperature. For the tests approximately 10 L of the material should be available, and a fresh material should be used for each individual test specimen. If such a quantity is not available, use a smaller shear cell. As a last resort shear tests have to be repeated on the same specimen, then before each test, the material should be well loosened.

A2.1.2 The flowability of a particulate solid is usually significantly dependent on its moisture content which at equilibrium depends on the ambient humidity. In view of the significant influence of moisture, the amount anticipated during actual storage and flow should be closely reproduced in the test specimen, for example, by equilibrating it to this humidity. To prevent moisture evaporation or adsorption, it is advisable to keep the test material in an airtight container, replacing the cover of the container between tests. To prevent inhomogeneities in water content, stir the material in the container regularly and, during the test, handle the specimen and the shear cells rapidly. Upon completion of time tests, recheck the moisture in the solid from the shear cells. Ideally, measurements should be made in an air-conditioned room with controlled humidity.

A2.1.3 The effect of particle size distribution is not as perplexing as it might appear. During the flow of a mass of mixed particle sizes, the large particles move bodily while the solid shears primarily across the fines. The coarse particles contribute little to the cohesion of the mass. Therefore, the flowability of the mass depends on the properties of the fines. The standard size shear tester is suitable for testing particulate solids with particle sizes of up to 5 % of the shear cell diameter. Coarser particles should be removed by hand. When removing the larger particles, it is necessary, in so far as possible, to retain the structure of the solid and the moisture content of the fines. If there is danger that by sieving the structure of the solid will be altered, spread the material gently on a tray and remove the larger particles by hand. Do not screen fibrous solids, whose strength is due to the interlocking of the fibers. Such solids are on the borderline of applicability of this test method. Take great care to ensure that the particles do not segregate between sample withdrawal and testing (for example, during transport coarse particles can segregate towards the surface of a material in a container and, if this surface material is taken for shear testing, it will have a lower shear strength). If tests are

repeated on the same specimen, take care not to lose fines, for example, by ventilation.

A2.1.4 The effect of temperature on the flowability of solids may be significant. Tests of such solids require either a shear tester and a consolidating bench which permit the adjustment and control of temperature, or a temperature-controlled room.

A2.1.5 The coupon of wall material should be flat, 120-mm square or larger and representative of the material on which the particulate solid will slide. Take care in that some types of wall materials have directional properties (for example, rolled steels). In such cases, orient the coupon of wall material in the same direction as in the actual equipment (hopper).

A2.1.6 The effect of vibration on the shear strength of particulate solids is not treated in this test method. However, since vibrations influence the shear strength of particulate solids to a considerable extent, take care that during measurements the shear cell is completely free of any vibration either from the force measuring stem driving mechanism, or from the test room.

### A2.2 Shear Cell Selection:

A2.2.1 When measuring the shear strength of particulate solids having bulk densities in the range from 300 to approximately 2400 kg/m<sup>3</sup>, use a standard size shear cell. For particulate solids with bulk densities below 300 kg/m<sup>3</sup>, or when performing shear tests at very low normal stress levels, a light metal shear cell is recommended.

A2.2.2 At higher preshear normal stress levels and with particulate solids having bulk densities above 2400 kg/m<sup>3</sup>, a shear cell of smaller diameter may be used if the available force transducer has insufficient range.

### A2.3 Equivalence Between Weights and Stresses:

A2.3.1 The results of shear tests are expressed in terms of stresses, that is, by the shear stress and the normal stress in the shear plane (plane between shear ring and base). However, cells are loaded by weights and therefore it is necessary to equate these weights to the corresponding stresses. This equivalence is given by the following equations:

$$\sigma = \frac{F_v}{A} = \frac{(m_B + m_R + m_L + m_W) g}{A} \quad (\text{A2.1})$$

where

$\sigma$  = normal stress, Pa,

$m_B$  = mass of particulate solid in the shear ring, kg,

$m_R$  = mass of the ring, kg,  
 $m_L$  = mass of the shear lid, kg,  
 $m_H$  = mass of the weight hanger, kg,  
 $m_W$  = mass of the weights, kg,  
 $g$  = acceleration due to gravity (9.81 m/s<sup>2</sup>), and  
 $A$  = cross-sectional area of the cell, m<sup>2</sup>.

$$\tau = \frac{F_s}{A} \quad (\text{A2.2})$$

where

$\tau$  = shear stress, Pa and

$F_s$  = shear force, N.

If weights are stacked directly on the lid when running shear tests without a hanger, the term  $m_H$  is omitted from Eq A2.1. Read the shear force,  $F_s$ , from the calibrated recorder.

A2.3.2 In this test method, the loading of shear cells by weights is expressed in the form of the normal stress in the shear plane. The operator of the shear cell must therefore derive from Eq A2.1 the corresponding weight of mass  $m_W$  which he places on the hanger. Masses of weights corresponding to preshear normal stresses may be rounded up to 1 kg if above 4 kg and to 0.5 kg if below 4 kg. Masses of weights corresponding to shear normal stresses may be rounded up to kilograms if above 6 kg, to 0.5 kg if between 2 and 6 kg, and to 0.1 kg if below 2 kg. This rounding up is used only for the selection of weights. From the total of the masses in question, calculate the normal stress to an accuracy of 10 Pa.

A2.3.3 In order to attain the required degree of accuracy, measure all weighed components to a precision of 1 g. Although weights are normally well within the required tolerance, it is advisable to check them on purchase. A recently calibrated balance is suitable for this. When plotting dependencies of  $\tau$  on  $\sigma$ , it is necessary to know the mass of powder above the plane of shear, which in Eq A2.1 is approximated by the amount of material in the ring,  $m_B$ . To measure this,

determine the total mass of particulate solid in the shear cell,  $m_M$ , by weighing the base, shear ring, shear lid, and material in the cell to the required 1-g precision after the shear test and from the known volumes of the ring and base, calculate the corresponding mass of material above the shear plane,  $m_B$ . This assumes that the contents of the cell are homogeneous with respect to density. For cohesive materials, the amount of material above the shear plane may be determined directly by weighing the shear ring and the powder in it after shear. As the actual shear plane does not usually coincide with the plane between the ring and base, direct measurement will probably give results that are different from those calculated from  $m_M$  and the volumes of the parts of the cell. For the same reason, directly determined values of  $m_B$  are likely to vary more from test to test.

A2.3.4 These discrepancies are likely to be insignificant for all tests, except for those at the lowest normal loads. For more free-flowing materials which are likely to fall out of the ring as it is lifted, the direct method cannot be used and the volumetric method is necessary. Particulate solids bulk densities,  $\gamma$ , should be calculated from  $m_M$  rather than direct weight of material in the ring as the differences in volume of solids from test to test due to differences in the position of the plane of shear will be much more significant with respect to the mass of particulate solids alone compared with the sum of the masses in Eq A2.1. As the base has an irregular inside shape, its volume is best determined by weighing it empty and then again when filled with water exactly to the top. Determine the volume of the ring from its dimensions.

A2.3.5 If the shear tester is used with several bases, rings, and shear lids, it is advisable to mark the parts so that the weights of those used in a particular test may be readily identified.

### A3. Optimization Procedure

A3.1 Trial tests which are performed with the aim of obtaining a critically consolidated specimen are called optimization. Optimization has to be repeated for each preshear normal stress level.

A3.2 Inspect the shear force-time record, which is equivalent to a shear stress-shear strain record, and depending upon the degree of compaction of the particulate solid produced by the applied weight of mass  $m_{WC}$ , three general types of shear force – shear strain curves may be obtained (shown by full lines in Fig. A3.1).

A3.3 If for the material under test the degree of compaction is insufficient, the shear force will increase continually during shear (Fig. A3.1, Curve 3). Such a specimen is said to be underconsolidated, and the bulk density in the shear zone increases during shear. If the degree of compaction is excessive, the shear force rises initially, passes through a maximum and then decreases (Curve 1). Such a specimen is said to be overconsolidated, and the bulk density in the shear zone is

thought to decrease after passing through a maximum.

A3.4 There is, however, a degree of compaction when the shear force rises initially, but having reached a certain value remains constant during the remainder of shear (Curve 2). Such a specimen is said to be critically consolidated, and that part of the test when the shear force is constant is called steady state flow. In such a specimen, the bulk density and shear stress in the shear zone remain constant during shear.

A3.5 It has been shown that for a given particulate solid at a given normal stress acting in the shear zone, the shear stress and the bulk density during steady state flow have unique values.

A3.6 Thus it can be seen that for a given  $m_{WP}$ , the plot of the shear force versus strain strongly depends on the original bulk density of the particulate solid in the cell which in turn is a result of the degree of compaction of the material during preparation of the specimen for shear testing.

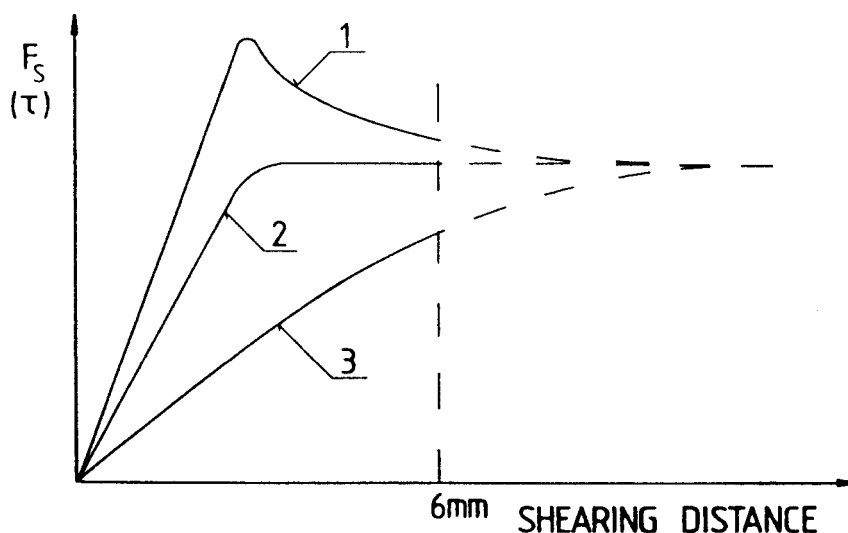


FIG. A3.1 Stress-Strain Curves for Over, Critically and Under Consolidated Samples

A3.7 The shear test therefore consists of two parts. The first of these is the preparation of a critically consolidated specimen and the attainment of steady state flow in the shear cell with a definite bulk density in the shear zone. This bulk density is defined by the values of the normal and shear stresses for steady state flow. In the second part of the test, an actual shear stress measurement is performed in which, for a selected value of the normal force,  $m_{ws} < m_{wp}$ , the necessary shear force for the material to yield is determined.

A3.8 To simplify the situation it is possible to imagine that if the shear cell was capable of shear through an infinite distance (for example, in an annular shear cell) then steady state flow could be attained simply by allowing the specimen to shear through a long enough distance. Underconsolidated samples would follow the full and then the dashed line marked 3 in Fig. A3.1. Overconsolidated samples would follow the full and then the dashed Curve 1 in Fig. A3.1. In both cases, the shear force would eventually attain the level corresponding to a critically consolidated specimen, as in Curve 2. (In reality, this is not necessarily the case since prolonged shear can by the attrition or orientation of particles in the shear zone lead to the formation of a single shear plane whose properties may be different from those of a shear zone.)

A3.9 The Jenike shear cell, however, is limited to a shear distance of approximately 6 mm, represented by the dashed vertical line in Fig. A3.1. Therefore steady state flow must be attained within a shear distance of up to 4 to 5 mm leaving the remaining distance for the actual shear test. For the specimen to attain steady state flow in such a short shear distance, the specimen must be close to critical consolidation prior to shear. The technique for obtaining steady state flow during a short shear distance, called consolidation, was developed by Jenike and consists of twisting and preshear.

A3.10 If the specimen is underconsolidated (Fig. A3.1, Curve 3), then prepare additional specimens in which the number of twists is increased stepwise to a maximum of 50.

For each number of twists selected, perform a shear test and inspect the stress-strain record until critical consolidation is found to have occurred. If after 50 twists the specimen is still underconsolidated, increase the consolidating normal stress applied during twisting by approximately  $0.5 \sigma_p$  increments. Shear specimens again after being twisted by up to 50 times. When a consolidating normal stress is found which produces an overconsolidated specimen (Curve 1), make a finer adjustment of the consolidating normal stress  $\sigma_{tw}$  and of the number of twists to obtain a critically consolidated specimen. Alternative techniques are sometimes required in order to obtain this desired result.

A3.11 If the specimen after the first test is overconsolidated, decrease the number of twists during twisting stepwise to a minimum of about 5 and, if the material is still found to be overconsolidated, reduce the consolidating normal stress. Again make a final adjustment of  $\sigma_{tw}$  and number of twists to obtain a critically consolidated specimen.

A3.12 The output from the stress-strain recorder need not be a smooth curve but may contain various irregularities due to the formation of shear planes. Some experience is necessary in interpreting the stress-strain records to identify the conditions for critical consolidation. It is helpful to obtain a distinctly underconsolidated specimen, a distinctly overconsolidated specimen, and then an intermediate condition for critical consolidation, tending toward the underconsolidated, never the overconsolidated state. With some materials it is very difficult to obtain overconsolidated samples. Therefore, it is necessary to judge the stress-strain curves from only underconsolidated and critically consolidated samples.

A3.13 The steady state shear stress should be attained after a shear distance not greater than three fourths of the total shear distance permitted by the amount of overlap between the rings in order to allow for further shear during the shear test itself.

A3.14 If the particulate solid specimen is prone to attrition,  $\sigma_{tw}$  should not be increased beyond  $2\sigma_p$ . If under these

conditions the specimen is still underconsolidated, perform twisting with  $\sigma_{tw} = 2\sigma_p$  and start preshear with a stack of weights on the hanger corresponding to a total normal force of  $2\sigma_p$ . As the shear force rises, remove weights one at a time, keeping pace with the rise of the shear force until only weights corresponding to  $\sigma_p$  remain on the hanger. At no time during preshear, however, should the recorded shear stress exceed that

finally determined for steady state flow.

A3.15 When testing some coarse materials, the front of the shear ring may rise somewhat (over 1 mm) during shear. In such cases, it is possible to manually press the front of the ring gently down.

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